

ANALYSIS AND DESIGN OF A DUPLEX BUILDING USING STAAD.PRO

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ABSTRACT: The aim of the project is to do analysis and design of a Duplex building using software. Design for beams, columns and footings were carried out using the Software STAAD Pro. Drawings were done using Auto-CAD. AutoCAD is a software application for 2D and 3D design and drafting and STAAD pro is a structural analysis and design computer program. It is one of the leading structural analysis and design software which supports more than 100 steel, concrete and timber design codes and has the largest worldwide user base. The building will be designed for static loads i.e., dead load live load and floor load etc. The detailed study of seismology will also be taken into consideration. The dead loads, live loads and seismic load will be determined using IS-456, IS-875, IS-13920, and IS-1893. Structural design was accomplished for reinforced concrete slabs, beams, columns and footings, based on this stress analysis and its resultants. Design drawings were prepared, and specifications for construction. In the final analysis of the project, we will be able to get find the detailing of reinforcement of the beams, columns footing from the STAAD Pro. the slabs form the manual designing. This reinforcement detailing will be clearly shown using the AutoCAD.

Keywords: -AutoCAD 2006, Dead load, Live load, Combination load, STAAD Pro.

INTRODUCTION

Building construction is the engineering deals with the construction of building such as Residential houses, Apartments and Duplex houses. In this report we are dealing with designing and analysis of Duplex building. Simply building can be define as an enclose space by walls with roof, food, cloth and the basic needs of human beings. In the early ancient times humans lived in caves, over trees or under trees, to protect themselves from wild animals, rain, sun, etc. as the times passed as humans being started living in huts made of timber branches. The shelters of those old have been developed now-a-days into beautiful houses. Rich people live in sophisticated condition houses.



Buildings are the important indicator of social progress of the county. Every human has desire to own comfortable homes on an average generally one spends his two-third life times in the houses. The security civic sense of the responsibility. These are the few reasons which are responsible that the person do utmost effort and spend hard earned saving in owning houses.

Now-a-days the house building is major work of the social progress of the county. Daily new techniques are being developed for the construction of houses economically, quickly and fulfilling the requirements of the community engineers and architects do the design work, planning and layout, etc., of the buildings. Draughts men are responsible for doing the drawing works of building as for the direction of engineers and architects. The draughtsman must know his job and should be able to follow the instruction of the engineer and should be able to draw the required drawing of the building, site plans and layout plans etc., as for the requirements.

A building frame consists of number of bays and story. A duplex building, multi-paneled frame is a complicated statically intermediate structure. A design of R.C building of G+1 story's frame work is taken up. The building in plan (67' x 36') consists of columns built monolithically forming a network. The size of building is 67' x 36' feet. The number of columns is 14. It is duplex structure.

The design is made using software on structural analysis design (Staad-pro). The building subjected to both the vertical loads as well as horizontal loads. The vertical load consists of dead load of structural components such as beams, columns, slabs etc. and live loads. The horizontal load consists of the wind forces thus building is designed for dead load, live load and wind load as per IS 875.

The building is designed as two-dimensional vertical frame and analyzed for the maximum and minimum bending moments and shear forces by trial-and-error methods as per IS 456-2000. The help is taken by software available in institute and the computations of loads, moments and shear forces and obtained from this software.

Statement of project

Salient features:

Utility of building	: Duplex House
No of stories	: G+1
Type of construction	: R.C.C framed structure
Types of walls	: brick wall

Geometric details:

Plinth	Height	: 1.5
Ground	floor	: 3m
Floor-to	-floor height	: 3m.



Concrete grade : M30

All steel grades : Fe415 grade

Literature review:

Method of analysis of statistically indeterminate portal frames:

- 1. Method of flexibility coefficients.
- 2. Slope displacements methods (iterative methods)
- 3. Moment distribution method
- 4. Kane's method
- 5. Cantilever method

Method of flexibility coefficients:

The method of analysis is comprises reducing the hyper static structure to a determinate structure form by:

Removing the redundant support (or) introducing adequate cuts (or) hinges.

Limitations:

It is not applicable for degree of redundancy>3

Slope displacement equations:

It is advantageous when kinematic indeterminacy <static indeterminacy. This procedure was first formulated by axle bender in 1914 based on the applications of compatibility and equilibrium conditions.

The method derives its name from the fact that support slopes and displacements are explicitly comported. Set up simultaneous equations is formed the solution of these parameters and the joint moment in each element or computed from these values.

Limitations:

A solution of simultaneous equations makes methods tedious for manual computations. This method is not recommended for frames larger than too bays and two storey's.

Iterative methods:

This method involves distributing the known fixed and moments of the structural member to adjacent members at the joints in order satisfy the conditions of compatibility.



Limitations of hardy cross method:

It presents some difficulties when applied to rigid frame especially when the frame is susceptible to side sway. The method cannot be applied to structures with intermediate hinges.

Kani's method:

This method over comes some of the disadvantages of hardy cross method. Kani's approach is similar to H.C.M to that extent it also involves repeated distribution of moments at successive joints in frames and continues beams. However, there is a major difference in distribution process of two methods. H.C.M distributes only the total joint moment at any stage of iteration.

The most significant feature of kani's method is that process of iteration is self-corrective.

Any error at any stage of iterations corrected in subsequent steps consequently skipping a few steps error at any stage of iteration is corrected in subsequent consequently skipping a few steps of iterations either by over sight of by intention does not lead to error in final end moments.

Advantages:

It is used for side way of frames.

Limitations:

The rotational of columns of any story should be functioning a single rotation value of same story.

The beams of story should not undergo rotation when the column undergoes translation. That is the column should be parallel.

Frames with intermediate hinges cannot be analysis.

Design of Duplex building:

General:

There are three paramount design principles which must be taken into consideration when designing a duplex or a triplex. These are:

Neighborhood context – the design should be consistent with, complement, or improve upon the design character of the immediate neighborhood.

variety in design – the duplex or triplex should incorporate design elements which help break up the mass of the building, provide for individuality in the design of each dwelling unit, and which are interesting to look at.

landscaping – the design should incorporate front yard landscaping that reinforces the individuality of the dwelling units, softens the edges of the building, and screens parking areas/driveways.



The main object of reinforced concrete design is to achieve a structure that will result in a safe economical solution.

The objective of the design is:

- 1. Foundation design
- 2. Column design
- 3. Beam design
- 4. Slab design

These all are designed under limit state method.

Limit state method:

The object of design based on the limit state concept is to achieve an acceptability that a structure will not become unserviceable in its life time for the use for which it is intended. It will not reach a limit state. In this limit state method, all relevant states must be considered in design to ensure a degree of safety and serviceability.

Limit state:

The acceptable limit for the safety and serviceability requirements before failure occurs is called a limit state.

Limit state of collapse:

This is corresponds to the maximum load carrying capacity.

Violation of collapse limit state implies failures in the source that a clearly defined limit state of structural usefulness has been exceeded. However, it does not mean complete collapse.

This limit state corresponds to:

- a) Flexural
- b) Compression
- c) Shear
- d) Torsion

Limit state of serviceability:

This state corresponds to development of excessive deformation and is used for checking member in which magnitude of deformations may limit the rise of the structure of its components.

a) Deflection

b) Cracking



c) Vibration

SOFTWARES

This project is mostly based on software and it is essential to know the details about these

software's.

List of software's used

- 1. Staad pro (v8i)
- 2. Staad foundations 5(v8i)
- 3. Auto cad



STAAD

Staad is powerful design software licensed by Bentley .Staad stands for structural analysis and design.

Any object which is stable under a given loading can be considered as structure. So first find the outline of the structure, whereas analysis is the estimation of what are the type of loads that acts on the beam and calculation of shear force and bending moment comes under analysis stage. Design phase is designing the type of materials and its dimensions to resist the load. This we do after the analysis.

To calculate shear force and bending moment of a complex loading beam it takes about an hour. So, when it comes into the building with several members it will take a week. Staad pro is a very powerful tool which does this job in just an hour's staad is a best alternative for high rise buildings.

Now-a-days most of the high-rise buildings are designed by staad which makes a compulsion for a civil engineer to know about this software.

This software can be used to carry rcc, steel, bridge, truss etc. according to various country codes.



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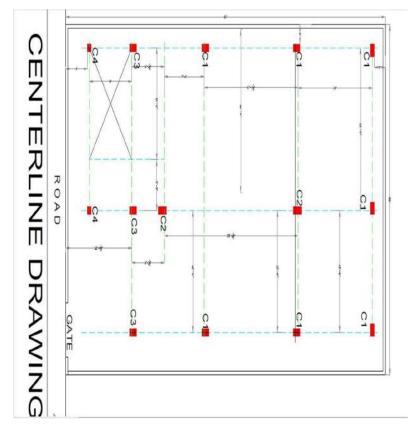


Fig. Center line plan

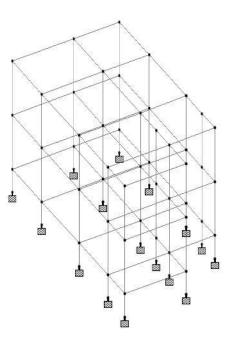


Figure Skeletal structure of the building



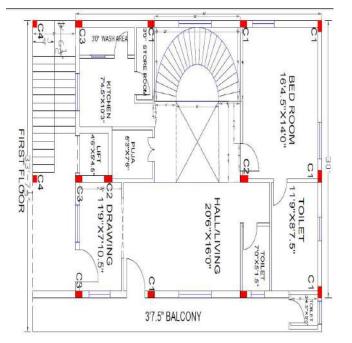


Figure Plan of the building

Load Calculation

Dead load calculation:

Weight=Volume x Density

Self-weight floor finish=0.12*25+1=3kN/m2

The above example shows a sample calculation of dead load.

Dead load is calculated as per IS 875 part 1

Live Loads:

In Staad we assign live load in terms of U.D.L .we has to create a load case for live load and select all the beams to carry such load. After the assignment of the live load the structure appears as shown below.

For our structure live load is taken as 2kN/m2 for design.

Live loads are calculated as per IS 875 part 2



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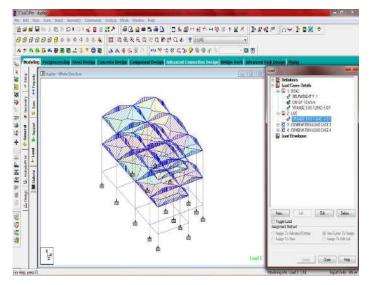


Fig 4.4.2a Diagram of live load

Floor load:

Floor load is calculated based on the load on the slabs. Assignment of floor load is done by creating a load case for floor load. After the assignment of floor load our structure looks as shown in the below figure.

The intensity of the floor load taken is: -2kN/m2 (-ve sign indicates that floor load is acting downwards.)

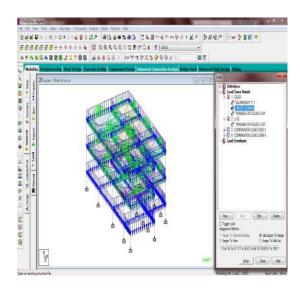


Fig 4.4.3.a Diagram of floor load

Load combinations:

All the load cases are tested by taking load factors and analyzing the building in different load combination as per IS456 and analyzed the building for all the load combinations and results are taken and maximum load combination is selected for the design.

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Load factors as per IS456-2000

Live Load	Dead Load
1.5	1.5

When the building is designed for both wind and seismic loads maximum of both is taken. Structure is analyzed by taking all the above combinations.

BEAMS

Beams transfer load from slabs to columns. beams are designed for bending.

In general, we have two types of beams: single and double. Similar to columns geometry and perimeters of the beams are assigned. Design beam command is assigned and analysis is carried out, now reinforcement details are taken.

Beam design:

A reinforced concrete beam should be able to resist tensile, compressive and shear stress induced in it by loads on the beam.

There are three types of reinforced concrete beams

1) Single reinforced beams

2) Double reinforced concrete

3) Flanged beams

Singly reinforced beams:

In singly reinforced simply supported beams steel bars are placed near the bottom of the beam where they are more effective in resisting in the tensile bending stress. I cantilever beams reinforcing bars placed near the top of the beam, for the same reason as in the case of simply supported beam.

Doubly reinforced concrete beams:

It is reinforced under compression tension regions. The necessity of steel of compression region arises due to two reasons. When depth of beam is restricted. The strength availability singly reinforced beam is in adequate. At a support of continuous beam where bending moment changes sign such as situation may also arise in design of a beam circular in plan.



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Figure shows the bottom and top reinforcement details at three different sections that are extracted from the Staad Pro. results of concrete designing.

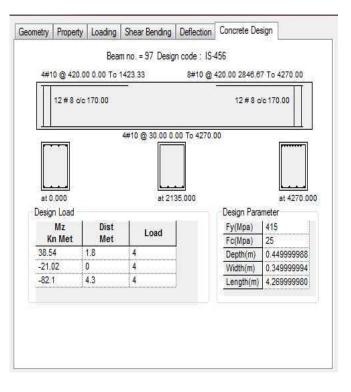


Fig 5.1.2a Diagram of the reinforcement details of beam

The following figure shows the deflection of a same beam:

etry Property	Loading	Shear Bending	Deflection	Concrete [)esign
		Be	am No = 97		
-		3	2.135		1
0-		3			7
-0.545					
		0	-1.280		-1.046
2					58
				Dist	Disp
flection				Dist m	Disp mm
flection Dist	Displ			m	mm
1999-1999-1999-1999-1999-1999-1999-199	Displ		0.00	m	a start and
Dist m 2.846666653	-1.204		0.00	m 10	mm
Dist m 2.846666653 3.202499985	mm -1.204 -1.128			m 10 n Type	mm 0.319
Dist m 2.846666653 3.202499985 3.558333317	mm -1.204 -1.128 -1.056		0.00	m 10	mm 0.319
Dist m 2.846666653 3.202499985	mm -1.204 -1.128 -1.056		0.00 Selection	m 10 Type 1:DE	mm 0.319 AD +
Dist m 2.846666653 3.202499985 3.558333317	mm -1.204 -1.128 -1.056 -1.017		0.00 Selection	m 10 n Type	mm 0.319 AD → n
Dist m 2.846666653 3.20249985 3.558333317 3.914166649 4.269999980	mm -1.204 -1.128 -1.056 -1.017 -1.046		0.00 Selection	m 10 1:DE 2:DE	mm 0.319 AD ← n © X Dir @ Y Dir
Dist m 2.846666653 3.202499985 3.558333317 3.914166649	mm -1.204 -1.128 -1.056 -1.017 -1.046		0.00 Selection	m 10 Type 1:DE	mm 0.319 AD ← n © X Dir @ Y Dir

Fig 5.1.2b Diagram for deflection of a beam

Deflection Value Table of beam no.2385
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Distance (m)	Displacement
0.000	-0.545
0.356	-0.72
0.712	-0.897
1.067	-1.055
1.423	-1.177
1.779	-1.253
2.135	-1.28
2.491	-1.26
2.847	-1.204
3.202	-1.128
3.558	-1.056
3.914	-1.017
4.270	-1.046

Figure shows the bending moment diagram of the particular beam (no. 2385), from the Staad Pro. result of analysis.

eometry	Prop	епу	Loading	Shear Bending	Deflection	Concrete	e Design	
				E	Beam No = 97			
21.1 10	-	38				3.15		82.10
Section F	escentite		Fy	Mz	Di	2.2	Fy kN	Mz kNm
	t		Fy kN	Mz kNm	·	1	kŇ	kNm
Dis	t	-44.3	kN		0.700	1		
Dis m	t 6653		kN 371	kNm	0.000	1	kN 19.941	kNm
Dis m 2.84666	t 6653 9985	-57.4	kN 371 410	kNm -15.013	0.000 Selec	n 5 ction Type	kN 9.941	kNm 21.025
Dis m 2.84666 3.20249	t 6653 9985 3317	-57.4 -69.	kÑ 371 410 119	kNm -15.013 3.135	0.000 Selec	n 5	kN 9.941	kNm
m 2.84666 3.20249 3.55833	t 6653 9985 3317 6649	-57.4 -69. -79.4	kÑ 371 410 119 499	kNm -15.013 3.135 25.686 52.167	0.000 Selec Loa	n 5 ction Type	kN 9.941 4:COMI Z	kNm 21.025



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Fig. 5.1.2c Diagram of Bending Moment of Beam

Section Forces Table

D	T	3.6
Distance	Fy	Mz
0	59.941	21.025
0.355833	50.89	1.267
0.711667	40.51	-15.034
1.0675	28.801	-27.405
1.423333	15.762	-35.373
1.779167	1.393	-38.465
2.135	-14.305	-36.207
2.490833	-30.002	-28.285
2.846667	-44.371	-15.013
3.2025	-57.41	3.135
3.558333	-69.119	25.686
3.914167	-79.499	52.167
4.27	-88.55	82.105

Due to huge output data, output of a sample beam is shown below.

	97 DE	SIGN RESUL			
M25		(Main)	Fe415 (Sec		
			i0.0 mm X 450	0 mm COVI	SR: 25.0 mm
SUMMAR	Y OF REIN	F. AREA (Sq.	mm)		
SECTION	0.0 mm	1067.5 mm	2135.0 mm	3202.5 mm	4270.0 mm
тор	301.08			301.08	611.06
REINF.	(Sq. mm)	(Sq mm)	(Sq. mm) (Sq mm) (S	iq. mm)
BOTTOM	0.00	301.08	301.08 30	1.08 0.6	0
			(Sq. mm)		Sq. mm)
SUMMAR	Y OF PRO	VIDED REIN	F. AREA		
SECTION	0.0 mm	1067.5 mm	2135.0 mm	3202.5 mm	4270.0 mm
TOP	4-10i	2-10i	2-1.01	4-10i	8-10i
REINT.	1 layer(s)	l layer(s)	1 layer(s)	l layer(s)	l layer(s)
BOTTOM	2-10	4-10%	4-101	4-101	2-105
REINF.		1 layer(s)		1 layer(s)	
SHEAR REINE			8i 2 legged 1		
KEENP.	@ 1/01	un c/c_12 1/4	/ mm e/c (@ 1)	vmnce @	170 mm c/c @ 170 mm
Shear desig	n results at	distance d (eff	fective depth) f	rom face of th	e support
Shear des	ign results a	t 565.0 mm	away from star	t support	
VY=	44.49 MCX =	3.04 LD=	4		
Provide	2 Legged 8	@ 170 mm c	fe		
Shaw day	in martin	+ 565.0 mm	away from end	manager	
Jacob des	and subsets 6	- Jose ann	4		



Column design:

A column may be defined as an element used primary to support axial compressive loads and with a height of a least three times its lateral dimension. The strength of column depends upon the strength of materials, shape and size of cross section, length and degree of proportional and dedicational restrains at its ends.

- A column may be classify based on deferent criteria such as
- 1) Shape of the section
- 2) Slenderness ratio (A=L+D)
- 3) Type of loading, land
- 4) Pattern of lateral reinforcement.

The ratio of effective column length to least lateral dimension is released to as slenderness ratio.

In our structure we have 3 types of columns.

Column with beams on two sides

Columns with beams on three sides

Columns with beams on four sides

So, we require three types of column sections. So, create three types of column sections and assign to the respective columns depending on the connection. But in this structure, we adopted same cross section throughout the structure with a rectangular cross section. In foundations we generally do not have circular columns if circular column is given it makes a circle by creating many lines to increase accuracy.

The column design is done by selecting the column and from geometry page assigns the dimensions of the columns. Now analyze the column for loads to see the reactions and total loads on the column by seeing the loads design column by giving appropriate parameters like:

- 1. Minimum reinforcement, max, bar sizes, maximum and minimum spicing.
- 2. Select the appropriate design code and input design column command for the entire column.
- 3. Now run analysis and select any column to collect the reinforcement details.

The following figure shows the reinforcement details of a beam in Staad Pro. The figure represents details regarding:

- 1. Transverse reinforcement
- 2. Longitudinal reinforcement

The type of bars to be used, amount of steel and loading on the column is represented in the below figure.



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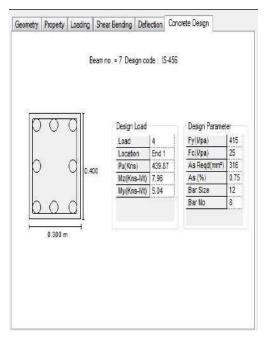


Fig. 6.3a Diagram of Reinforcement Detailing of Column

Staad Pro. Output Result:

Due to very huge and detailed explanation of Staad output for each and every column we have shown a column design result below showing the amount of load, moments, amount of steel required, section adopted etc. The main problem with Staad is it takes all columns also as beams initially before design and continue the same.so here output of column which is actually 7th beam as most of beams are used in drawing the plan.

Output for column:

COLUMN NO.7 DESIGN RESULTS
M25 Fe415 (Main) Fe415 (Sec.)
LENGTH: 1500.0 mm CROSS SECTION: 300.0 mm X 400.0 mm COVER: 40.0 mm
** GUIDING LOAD CASE: 4 END JOINT: 1 SHORT COLUMN REQD. STEEL AREA 🚅 315.60 Sq mm.
REQD_CONCRETE AREA: 39450.56 Sq.mm.
MAIN REINFORCEMENT: Provide 8 - 12 dia. (0.75%, 904.78 Sq.mm.)
(Equally distributed)
TIE REINFORCEMENT: Provide 8 mm dia. rectangular ties @ 190 mm c/c
Puz. 1444.68 Muz1: 71.40 Muy1: 52.60
INTERACTION RATIO: 0.21 (as per Cl. 39.6, IS456:2000)
SECTION CAPACITY BASED ON REINFORCEMENT PROVIDED (KNS-MET)
WORST LOAD CASE: 3
END JOINT 4 Puz: 1621.43 Muz: 94.79 Muy: 68.10 IR: 0.35

The following figure shows the bending moment diagram of same column.

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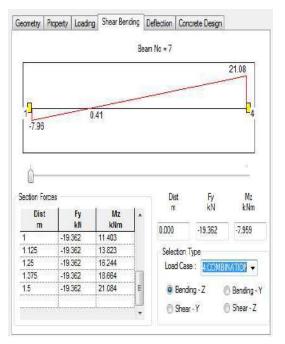


Fig 6.4a Deflection of column

Section Forces Table of Column

Distance(m)	Fy(kN)	Mz(kNm)
0	-19.362	-7.959
0.125	-19.362	-5.539
0.25	-19.362	-3.118
0.375	-19.362	-0.698
0.5	-19.362	1.722
0.625	-19.362	4.142
0.75	-19.362	6.563
0.875	-19.362	8.983
1	-19.362	11.403
1.125	-19.362	13.823
1.25	-19.362	16.244
1.375	-19.362	18.664
1.5	-19.362	21.084



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CONCLUSION & FUTURE SCOPE

CONCLUSION

To bring about a conclusion to this project a few important points are highlighted that has tremendously made it simple to design the building with the help of STAAD Pro with the use of this software it has been possible to check the axial force, shear force, torsion and bending moment of the building structure. It makes it easy to design and also saves a lot of time as designing manually consumes unnecessary amount of time and energy. This project has enabled to emerge out new ideas in the field of designing multistoried buildings.

In this project using this software we have got the structural designing of the whole building which means the reinforcement details of the each and every member in the structure. Here we have determined the reinforcement details of beams, columns from the STAAD Pro. and for slabs and stair case designed manually.

This project presents critical review of recommendations of well-established codes regarding design and detailing aspects of beam column slabs, staircase and footing. The codes of practice considered are IS 456, SP 16. Stress analysis was conducted using STAAD Pro software. Structural design was accomplished for reinforced concrete slabs, beams, columns and footings, based on this stress analysis and its resultants.

Future Scope:

For the extension of the project, we can also design the same building in Staad Pro. using the earthquake analysis and designing and also can go for the ductile detailing as per codes IS 1893 Part-1 and IS 13920 respectively.

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